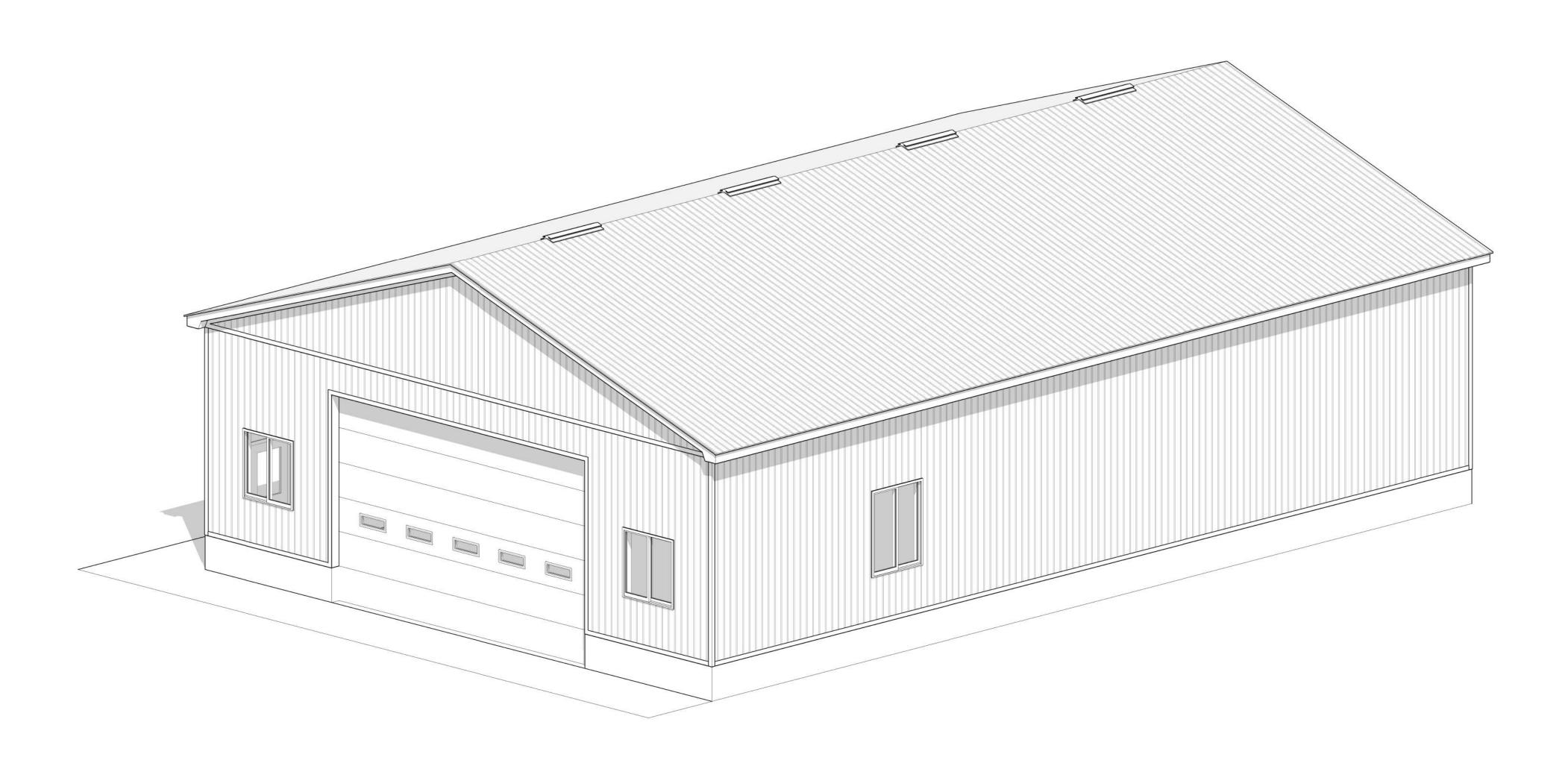
TONY HENDRIKX **RESIDENTIAL ACCESSORY BUILDING** GRAND BEND, ON



DRAWINGS

- TITLE SHEET **S-0**
- ELEVATIONS S-1
- FOUNDATION PLAN SECTIONS & DETAILS S-2
- S-3 **GROUND FLOOR PLAN - SECTIONS & DETAILS**
- **ROOF FRAMING PLAN SECTION DETAILS** S-4
- **SECTIONS & DETAILS** S-5
- **GENERAL NOTES SITE LOCATION PLAN** S-6

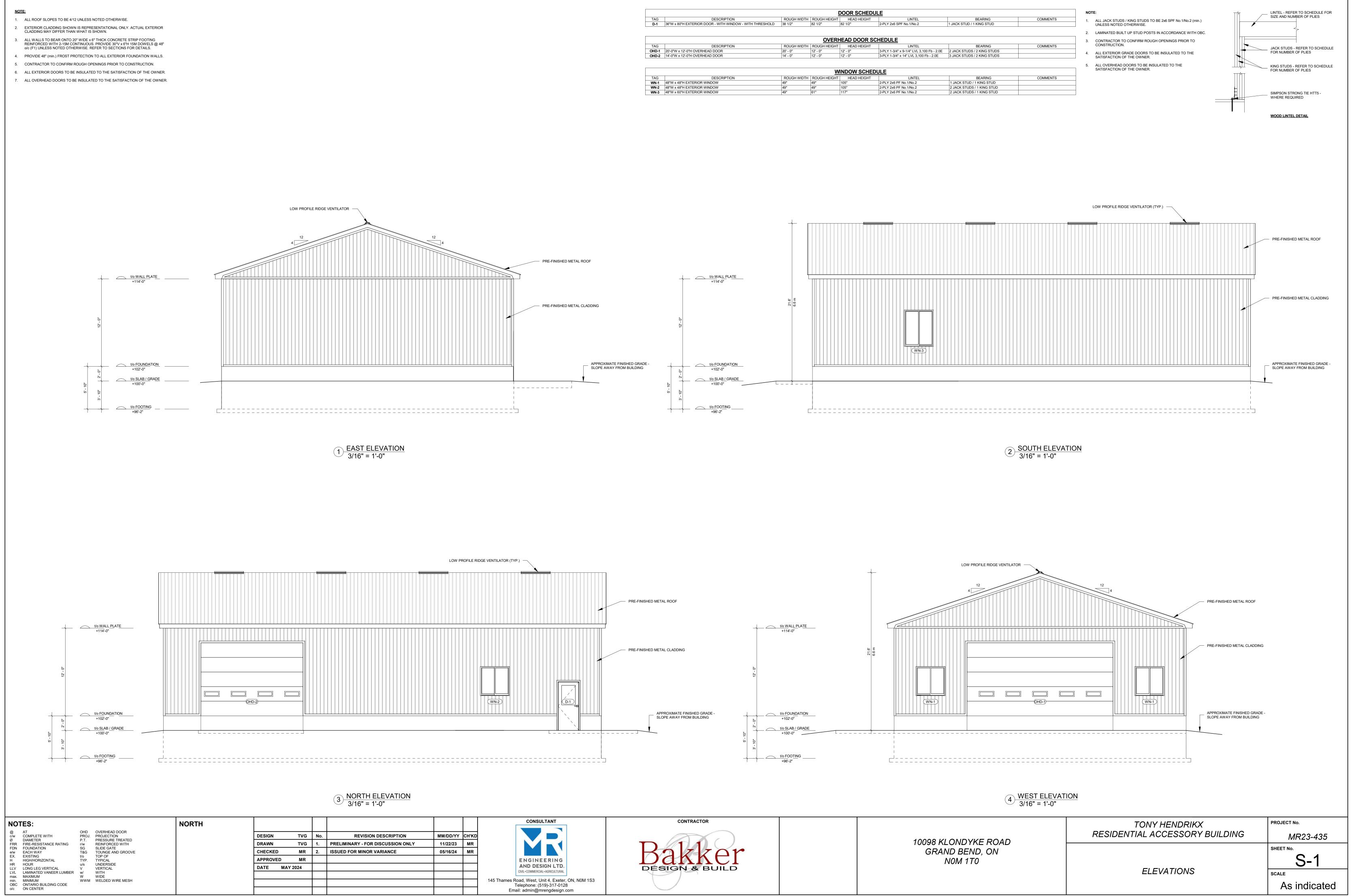
NOTES:	NORTH					
		DESIGN	TVG	No.	REVISION DESCRIPTION	MM/DD/YY
		DRAWN	TVG	1.	PRELIMINARY - FOR DISCUSSION ONLY	11/22/23
		CHECKE	D MR	2.	ISSUED FOR MINOR VARIANCE	05/16/24
		APPROVE	ED MR			
		DATE	MAY 2024			



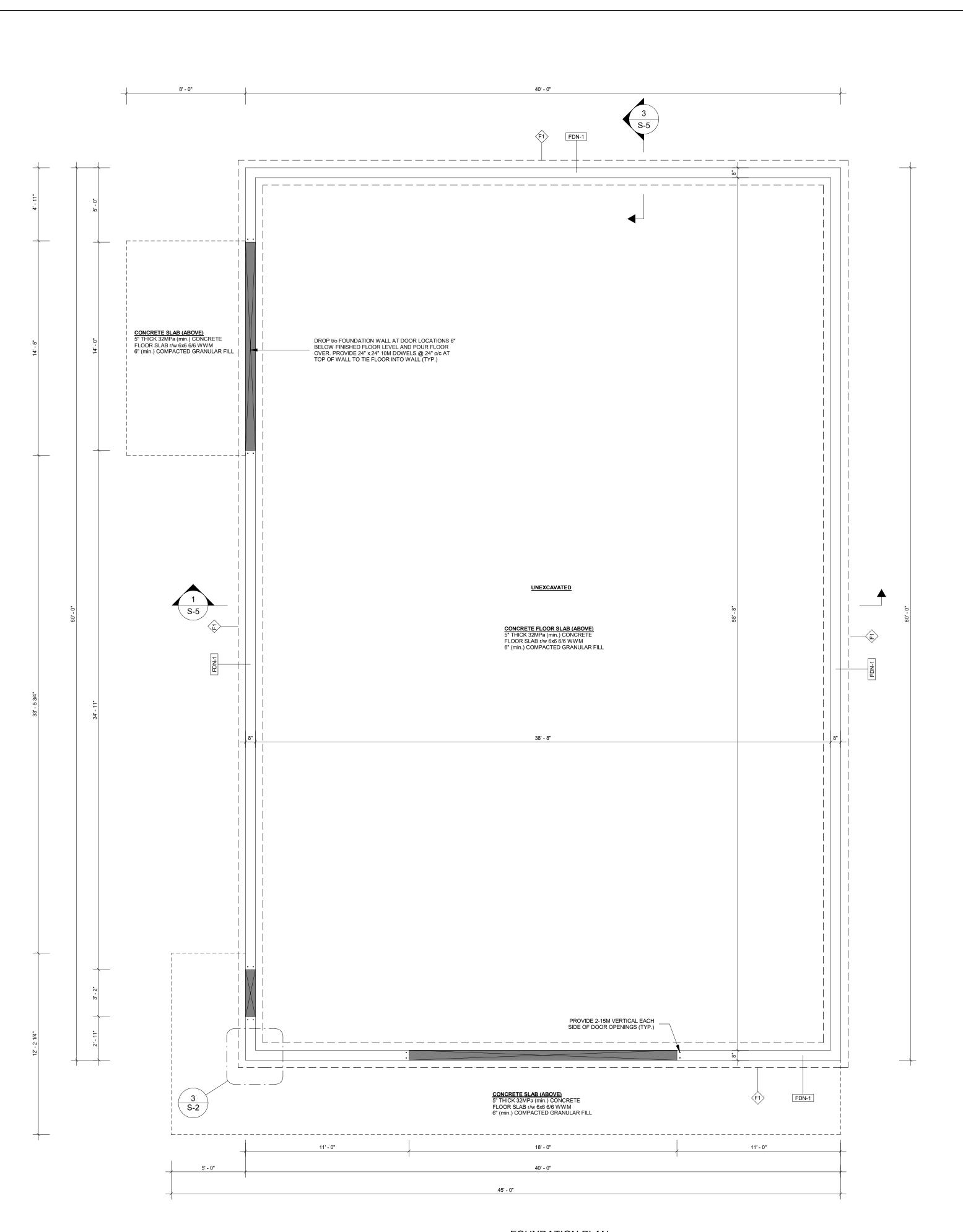


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LONDYKE ROAD	TONY HENDRIKX RESIDENTIAL ACCESSORY BUILDING	ргојест №. <i>MR23-435</i>
ND BEND, ON NOM 1T0		SHEET NO. S-0
	TITLE SHEET	SCALE



			<u> </u>	DOOR SCHEDL	ILE	
TAG	DESCRIPTION	ROUGH WIDTH	ROUGH HEIGHT	HEAD HEIGHT	LINTEL	Т
D-1	36"W x 80"H EXTERIOR DOOR - WITH WINDOW - WITH THRESHOLD	38 1/2"	82 1/2"	82 1/2"	2-PLY 2x6 SPF No.1/No.2	1
			OVER		CHEDULE	
TAG	DESCRIPTION	ROUGH WIDTH	ROUGH HEIGHT	HEAD HEIGHT		Т
OHD-1	20'-0"W x 12'-0"H OVERHEAD DOOR	20' - 0"	12' - 0"	12' - 0"	3-PLY 1-3/4" x 9-1/4" LVL 3,100 Fb - 2.0E	2
OHD-2	14'-0"W x 12'-0"H OVERHEAD DOOR	14' - 0"	12' - 0"	12' - 0"	3-PLY 1-3/4" x 14" LVL 3,100 Fb - 2.0E	3
			<u>w</u>	INDOW SCHED	OULE	
TAG	DESCRIPTION	ROUGH WIDTH	ROUGH HEIGHT	HEAD HEIGHT		_
TAG WN-1	DESCRIPTION 48"W x 48"H EXTERIOR WINDOW	ROUGH WIDTH				
-			ROUGH HEIGHT	HEAD HEIGHT	LINTEL	



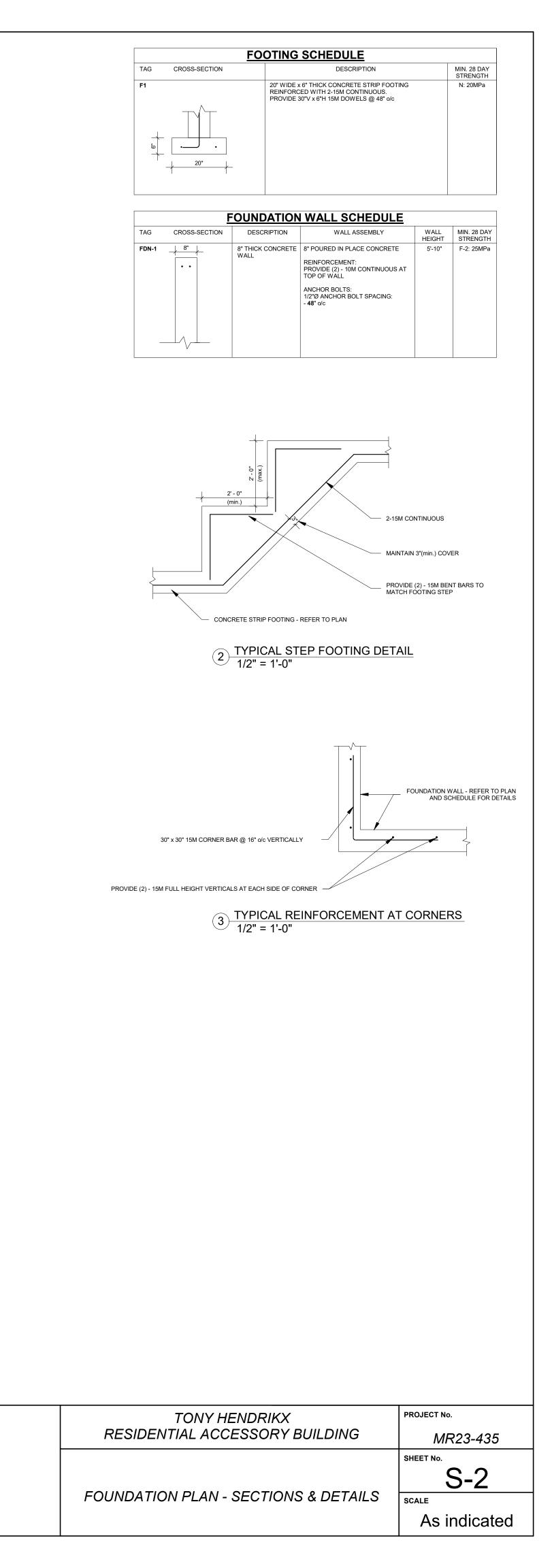
N	IOTES:								
				NORTH					
	w COMPLETE WITH	OHD PROJ.	OVERHEAD DOOR PROJECTION		DESIGN	TVG	No.	REVISION DESCRIPTION	MM/DD/Y
	RR FIRE-RESISTANCE RATING	P.T. r/w	PRESSURE TREATED REINFORCED WITH		DRAWN	TVG	1.	PRELIMINARY - FOR DISCUSSION ONLY	11/22/23
е	DN FOUNDATION /w EACH WAY	SG T&G	SLIDE GATE TOUNGE AND GROOVE	۲	CHECKE	D MR	2.	ISSUED FOR MINOR VARIANCE	05/16/24
F	X. EXISTING I HIGH/HORIZONTAL	t/o TYP.	TOP OF TYPICAL	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	APPROV	ED MR			
L	IR HOUR LV LONG LEG VERTICAL	u/s V	UNDERSIDE VERTICAL		DATE	MAY 2024			
	VL LAMINATED VANEER LUMBER nax. MAXIMUM	w/ W	WITH WIDE						
	nin. MINIMUM DBC ONTARIO BUILDING CODE	WWM	WELDED WIRE MESH						
0	/c ON CENTER								



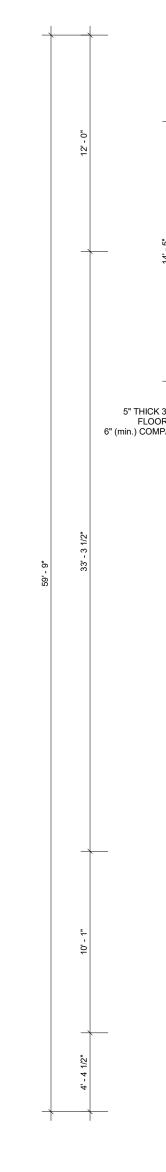


10098 KLONDYKE ROAD GRAND BEND, ON N0M 1T0

1 FOUNDATION PLAN 1/4" = 1'-0"



		NOT	TE:						
TAG D-1	DESCRIPTION 36"W x 80"H EXTERIOR DOOR - WITH WINDOW - WITH THRESHOLD		ROUGH HEIGHT 82 1/2"	HEAD HEIGHT 82 1/2"	LINTEL 2-PLY 2x6 SPF No.1/No.2	BEARING 1 JACK STUD / 1 KING STUD	COMMENTS	1.	ALL JACK STUDS / KING STUDS TO BE 2x6 SPF No.1/No.2 (min.) UNLESS NOTED OTHERWISE.
								2.	LAMINATED BUILT UP STUD POSTS IN ACCORDANCE WITH OBC.
				HEAD DOOR SO		_		3.	CONTRACTOR TO CONFIRM ROUGH OPENINGS PRIOR TO CONSTRUCTION.
TAG	DESCRIPTION	ROUGH WIDTH	ROUGH HEIGHT	HEAD HEIGHT	LINTEL	BEARING	COMMENTS		CONSTRUCTION.
OHD-1	20'-0"W x 12'-0"H OVERHEAD DOOR	20' - 0"	12' - 0"	12' - 0"	3-PLY 1-3/4" x 9-1/4" LVL 3,100 Fb - 2.0E	2 JACK STUDS / 2 KING STUDS		1	ALL EXTERIOR GRADE DOORS TO BE INSULATED TO THE
OHD-2	14'-0"W x 12'-0"H OVERHEAD DOOR	14' - 0"	12' - 0"	12' - 0"	3-PLY 1-3/4" x 14" LVL 3,100 Fb - 2.0E	3 JACK STUDS / 2 KING STUDS		4.	SATISFACTION OF THE OWNER.
								5.	ALL OVERHEAD DOORS TO BE INSULATED TO THE SATISFACTION OF THE OWNER.
			<u>w</u>	INDOW SCHEE	DULE				
TAG	DESCRIPTION	ROUGH WIDTH	ROUGH HEIGHT	HEAD HEIGHT	LINTEL	BEARING	COMMENTS		
WN-1	48"W x 48"H EXTERIOR WINDOW	49"	49"	105"	2-PLY 2x6 PF No.1/No.2	1 JACK STUD / 1 KING STUD			
WN-2	48"W x 48"H EXTERIOR WINDOW	49"	49"	105"	2-PLY 2x6 PF No.1/No.2	2 JACK STUDS / 1 KING STUD			
WN-3	48"W x 60"H EXTERIOR WINDOW	49"	61"	117"	2-PLY 2x6 PF No.1/No.2	2 JACK STUDS / 1 KING STUD			



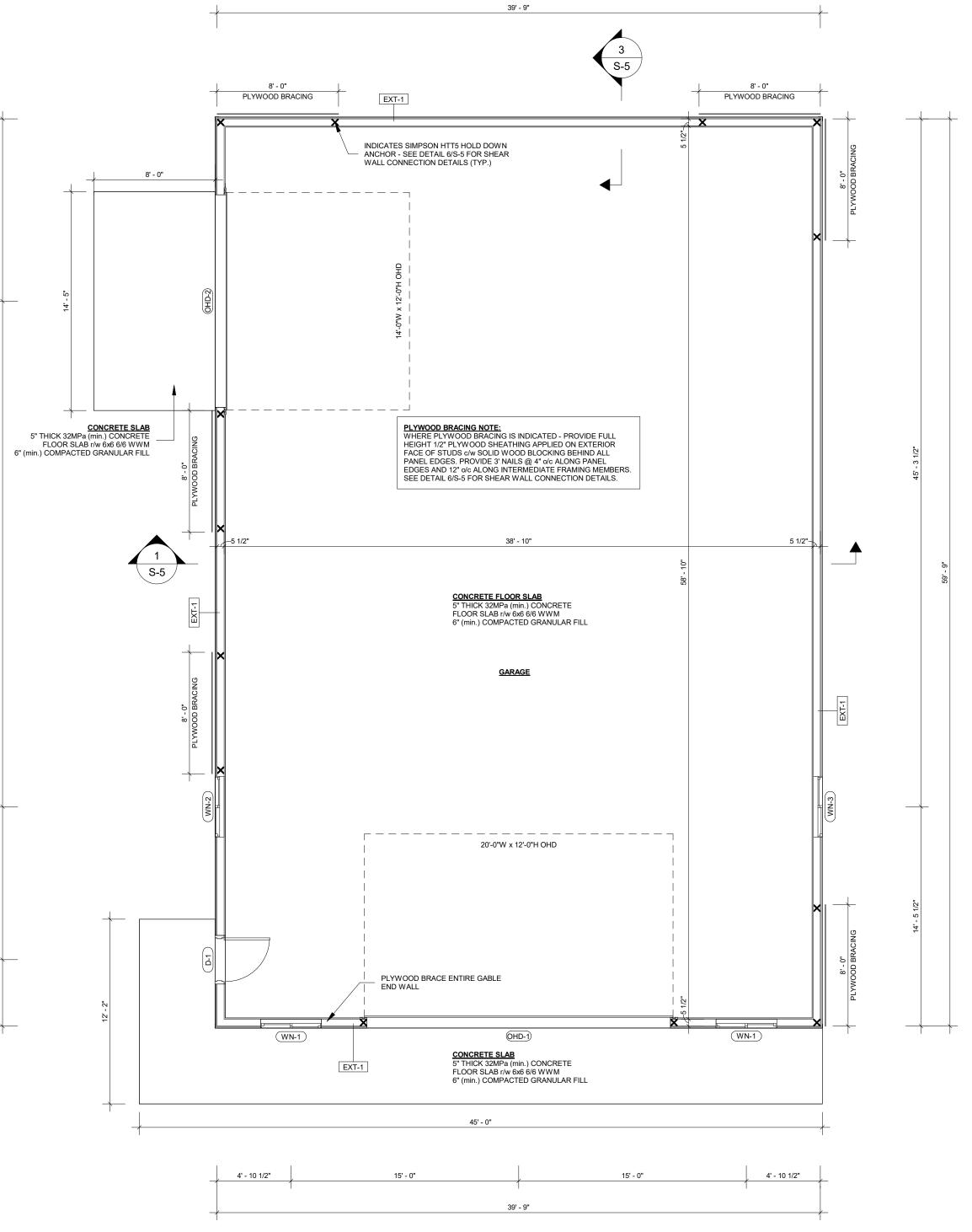
NO	TES:			NORTH					
@ c/w	AT COMPLETE WITH	OHD PROJ.	OVERHEAD DOOR PROJECTION		DESIGN	TVG	No.	REVISION DESCRIPTION	MM/DD/YY
Ø FRR	DIAMETER FIRE-RESISTANCE RATING	P.T. r/w	PRESSURE TREATED REINFORCED WITH		DRAWN	TVG	1.	PRELIMINARY - FOR DISCUSSION ONLY	11/22/23
FDN e/w	FOUNDATION EACH WAY	SG T&G	SLIDE GATE TOUNGE AND GROOVE		CHECKED	MR	2.	ISSUED FOR MINOR VARIANCE	05/16/24
EX. H	EXISTING HIGH/HORIZONTAL	t/o TYP.	TOP OF TYPICAL	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	APPROVE	D MR			
HR LLV	HOUR LONG LEG VERTICAL	u/s V	UNDERSIDE VERTICAL		DATE	MAY 2024			
LVL max.	LAMINATED VANEER LUMBER MAXIMUM	w/ W	WITH WIDE						
min. OBC	MINIMUM ONTARIO BUILDING CODE	WWM	WELDED WIRE MESH						
o/c	ON CENTER								

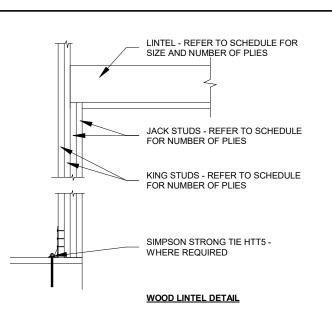




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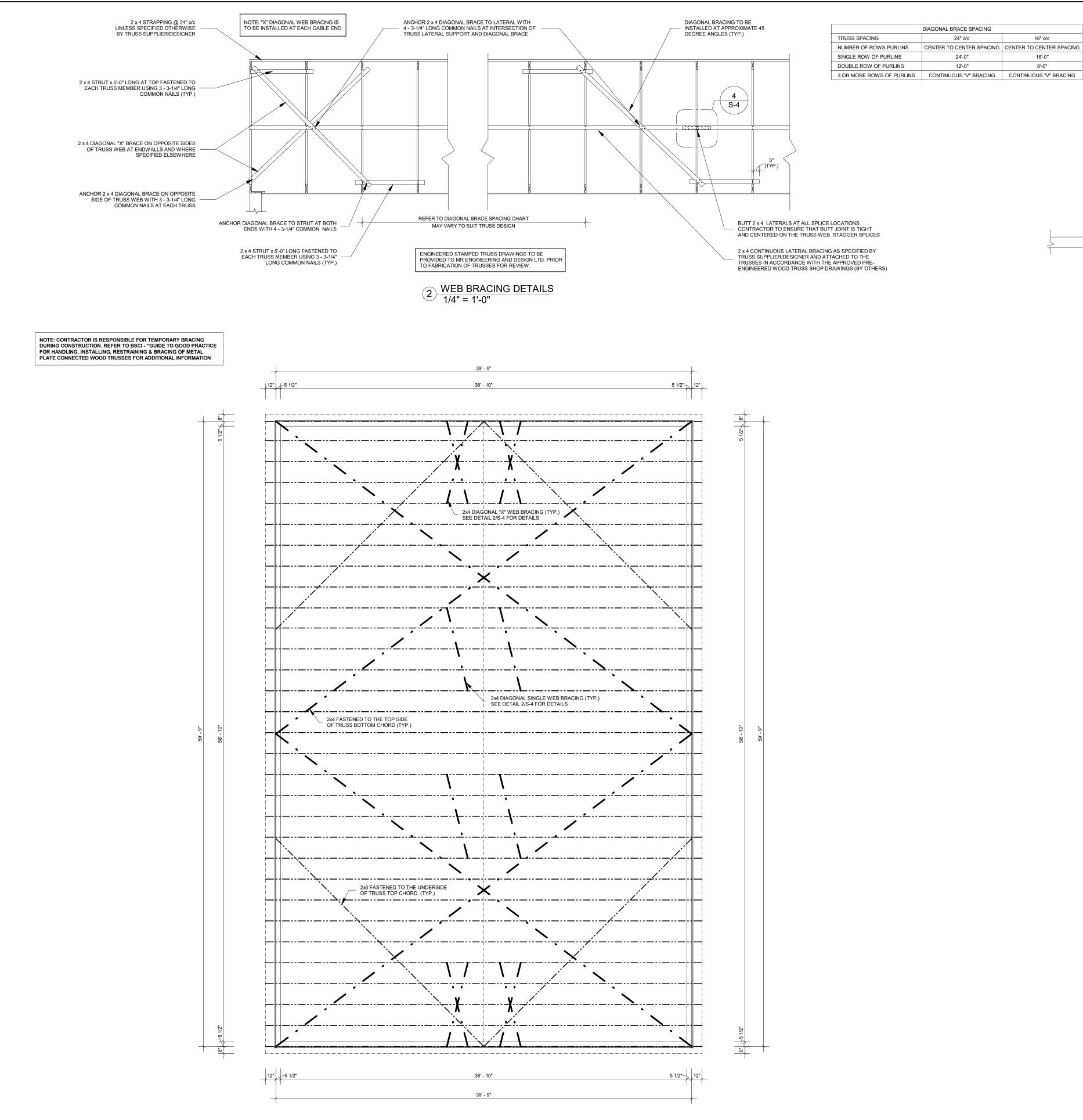
1 GROUND FLOOR PLAN 3/16" = 1'-0"





	WALL SCHEDULE						
TAG	CROSS-SECTION	DESCRIPTION	WALL ASSEMBLY	WALL HEIGHT			
EXT-1		2x6 FRAMED WALL	EXTERIOR 29ga. HI-RIB STEEL c/w SCREW FASTENERS 1x4 WOOD STRAPPING @ 24" o/c TYVEK AIR BARRIER (OR EQUIVALENT) 2x6 SPF No. 1/No. 2 WOOD STUDS @ 16" o/c SOLID WOOD BLOCKING @ 48" o/c VERTICALLY PROVIDE 12" WIDE STRIP OF 3/4" THICK PLYWOOD AROUND PERIMETER OF EXTERIOR WALLS WHERE THERE IS NO PLYWOOD BRACING TO CONNECT STUDS TO TOP / BOTTOM PLATE. FASTEN PLYWOOD TO STUDS AND TOP / BOTTOM PLATE USING 3" COMMON WIRE NAILS @ 4" o/c	12' - 0"			

LONDYKE ROAD	TONY HENDRIKX	PROJECT No.
	RESIDENTIAL ACCESSORY BUILDING	MR23-435
ND BEND, ON		SHEET No.
NOM 1TO		S-3
	GROUND FLOOR PLAN - SECTIONS & DETAILS	SCALE
		As indicated



TRUSS BRACING NOTE:

TRUSSES TO BE DESIGNED FOR THE LOADS NOTED ON THE STRUCTURAL DRAWINGS.

NATIONAL BUILDING CODE AS APPLICABLE.

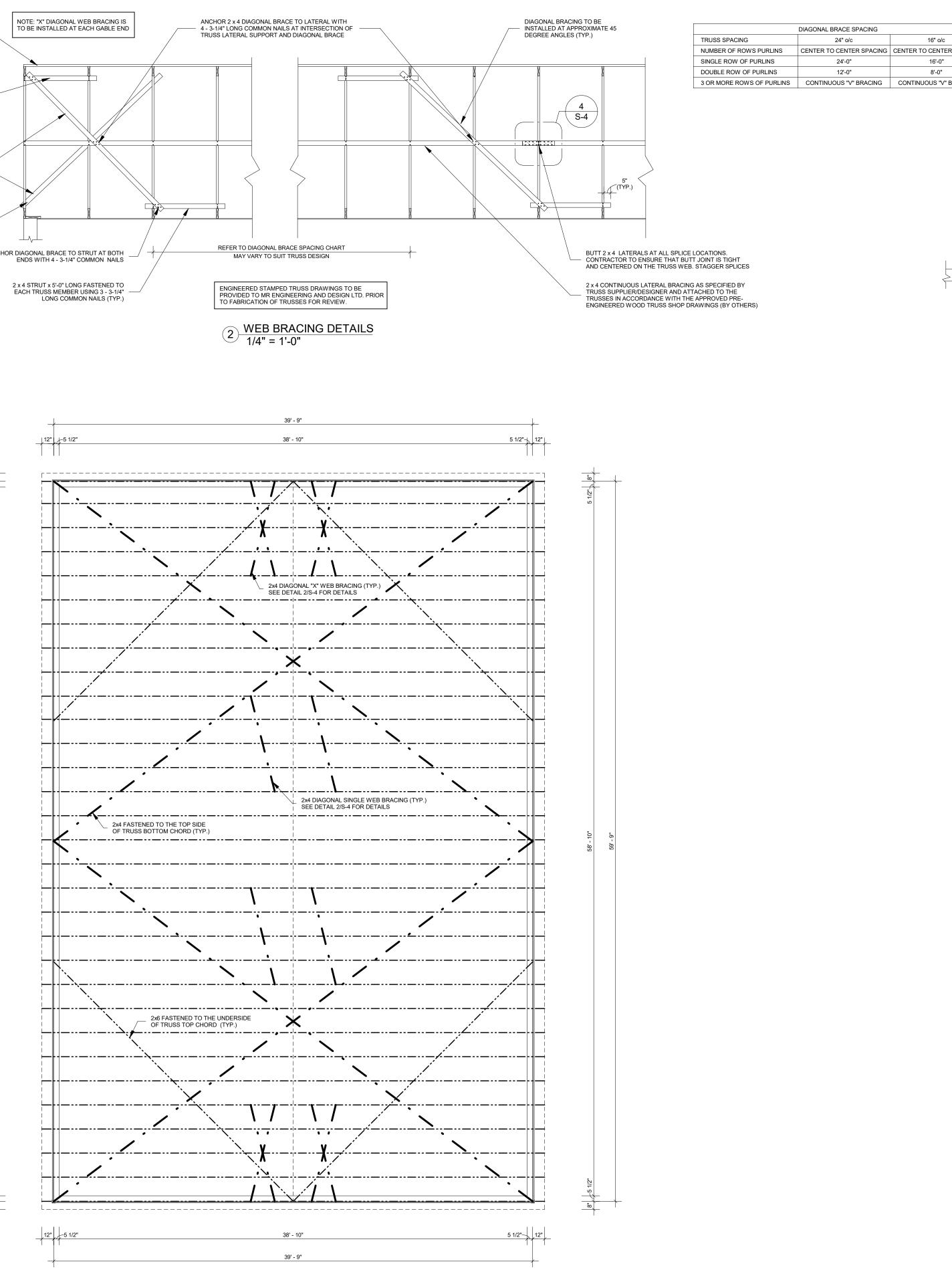
THE TRUSS DESIGNER SHALL ACCOUNT FOR UNBALANCED LOADING CONDITIONS, VALLEY LOADS, SNOW SHADOWS, AND WIND UPLIFT IN ACCORDANCE WITH THE ONTARIO BUILDING CODE, AND THE

BRACE SPACING AND LOADS ON DIAGONAL BRACING TO BE BASED ON A CUMULATIVE FORCE OF 2.0% OF THE WEB COMPRESSION. VERIFY LOADING WITH TRUSS SHOP DRAWINGS.

ALL PERMANENT CONTINUOUS LATERAL BRACING TO BE INSTALLED ON WEB MEMBERS AS INDICATED BY THE TRUSS SUPPLIER/DESIGNER. SIZE AND LOCATION OF LATERAL BRACES AND CONNECTION TO THE TRUSSES SHALL BE IN ACCORDANCE WITH THE APPROVED PRE-ENGINEERED WOOD TRUSS SHOP DRAWINGS (BY OTHERS).

DIAGONAL BRACING MUST BE PROVIDED ON ALL WEB MEMBERS THAT REQUIRE LATERAL BRACING. DIAGONAL BRACING SHALL BE SPACED AS NOTED IN DETAIL 2 ON THIS DRAWING AND X-BRACING SHALL BE PROVIDED AT ENDWALLS.

ADDITIONAL TRUSS BRACING MAY BE REQUIRED TO ACCOMMODATE THE TRUSS DESIGN AFTER SHOP DRAWINGS ARE REVIEWED.



	TES:			NORTH						
@ c/w	AT COMPLETE WITH	OHD PROJ.	OVERHEAD DOOR PROJECTION			DESIGN	TVG	No.	REVISION DESCRIPTION	MM/DD/YY
Ø FRR	DIAMETER FIRE-RESISTANCE RATING	P.T. r/w	PRESSURE TREATED REINFORCED WITH			DRAWN	TVG	1.	PRELIMINARY - FOR DISCUSSION ONLY	11/22/23
FDN e/w	FOUNDATION EACH WAY	SG T&G	SLIDE GATE TOUNGE AND GROOVE		Υ	CHECKED	MR	2.	ISSUED FOR MINOR VARIANCE	05/16/24
EX. H	EXISTING HIGH/HORIZONTAL	t/o TYP.	TOP OF TYPICAL	-	- 7 -	APPROVE	D MR			
HR LLV	HOUR LONG LEG VERTICAL	u/s V	UNDERSIDE VERTICAL			DATE	MAY 2024			
LVL max.	LAMINATED VANEER LUMBER	w/ W	WITH WIDE							
min. OBC	MINIMUM ONTARIO BUILDING CODE	WWM	WELDED WIRE MESH							
o/c	ON CENTER									

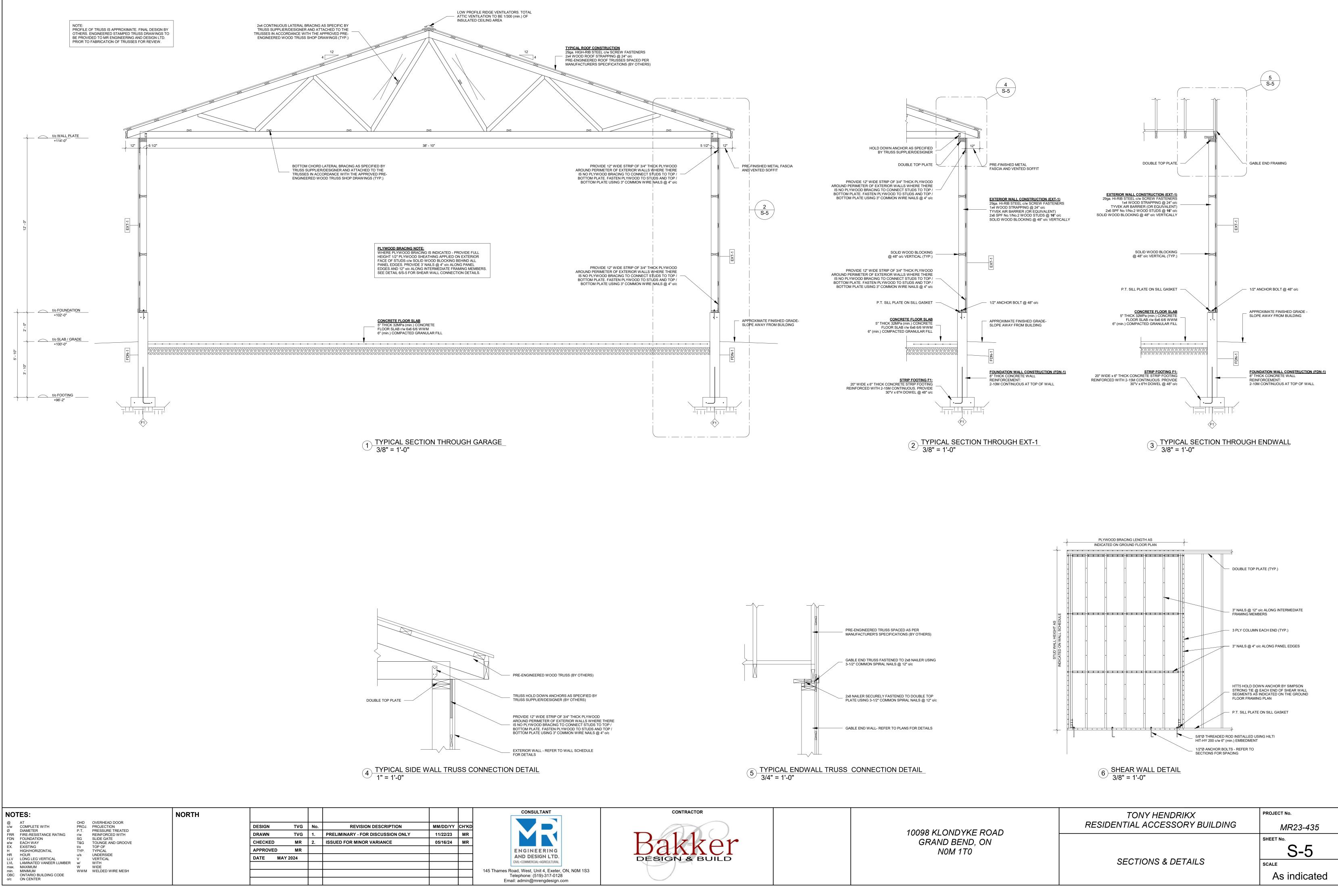
1 ROOF FRAMING PLAN 3/16" = 1'-0"





10098 KLONI GRAND E NOM

3 <u>TYPICAL BUT</u> 1 1/2" = 1'-0"	TRUSS MEMBER	
	STAGGER BUTT JOINTS WITH OTHER CONTINUOUS LATERAL BRACING TRUSS BUTT JOINT (UNDER WOOD SCAB) CONTINUOUS LATERAL BRACING AS SPECIFIED BY TRUSS SUPPLIER/DESIGNER	
	MINIMUM 24" LONG 2 x 4 SCAB BLOCK CENTERED OVER CONTINUOUS LATERAL BRACING. ATTACH SCAB USING 8 - 3-1/4" COMMON WIRE NAILS EACH SIDE OF SPLICE TRUSS MEMBER	
	MANUFACTURER'S SPECIFICATIONS (BY OTHERS)(TYP.)	
5 <u>ALTERNATIVE</u> 1/2" = 1'-0"	E CONTINUOUS LATERAL BRACE SPLICE CONNECTION	
$ \begin{array}{c} $	E CONTINUOUS LATERAL BRACE SPLICE CONNECTION	
$(5) \frac{\text{ALTERNATIVE}}{1/2" = 1'-0"}$	E CONTINUOUS LATERAL BRACE SPLICE CONNECTION	



GENERAL NOTES:

- 1. ALL DIMENSIONS ARE IN IMPERIAL UNITS UNLESS NOTED OTHERWISE.
- 2. THE CONTRACTOR IS TO CHECK AND VERIFY ALL DIMENSIONS ON THE STRUCTURAL DRAWINGS WITH OTHER DRAWINGS. ANY DISCREPANCIES OR ERRORS MUST BE REPORTED TO THE ENGINEER PRIOR TO COMMENCING WORK.
- 3. DO NOT SCALE DRAWINGS
- 4. FEATURES OF CONSTRUCTION NOT FULLY SHOWN ARE OF THE SAME CHARACTER AS THOSE NOTED FOR SIMILAR CONDITIONS
- 5. STRUCTURAL DESIGN IS BASED ON THE LATEST EDITION OF THE NATIONAL BUILDING CODE OF CANADA AND THE ONTARIO BUILDING CODE, SPECIFICALLY, DIV. B - PARTS 4 & 9 OF THE 2012 ONTARIO BUILDING CODE.
- 6. THE CONTRACTOR IS RESPONSIBLE FOR ARRANGING FOR THE INSPECTION OF STRUCTURAL REINFORCING AND STRUCTURAL STEEL FOR
- ALIGNMENT, BOLTS AND WELDED CONNECTIONS AND FOR THE PROMPT SUBMISSION OF ALL REPORTS TO THE PROJECT ENGINEER. THE CONTRACTOR IS RESPONSIBLE TO PROVIDE ADEQUATE PROVISIONS FOR TEMPORARY BRACING DURING CONSTRUCTION TO RESIST THE APPLIED LOADS THAT ARE INDICATED IN THESE DRAWINGS AND SUBSEQUENT DRAWINGS BY OTHERS.

EXCAVATION & BACKFILL

- 1. REMOVE ALL TOPSOIL AND DELETERIOUS MATERIAL FROM BELOW THE EXTENTS OF THE EXCAVATION.
- 2. SLOPE GRADE TO DRAIN AWAY FROM BUILDINGS.
- 3. FOUND FOOTINGS WHICH ARE EXPOSED TO FREEZING WEATHER A MINIMUM OF 4'-0" BELOW FINISHED GRADE UNLESS SPECIFIED OTHERWISE. 4. FOUND ALL FOOTINGS ON NATURALLY CONSOLIDATED, UNDISTURBED SOIL CAPABLE OF SAFELY SUSTAINING 150 kPa (3,100 psf) AT SERVICEABILITY LIMIT STATES (SLS) CORRESPONDING TO DENSE OR COMPACT SAND OR GRAVEL (TABLE 9.4.4.1. OBC).
- 5. NO FOOTINGS SHALL BE POURED UNTIL THE BUILDING DEPARTMENT HAS APPROVED THE FOUNDATION CONDITIONS.
- 6. ALL STANDING WATER SHALL BE REMOVED FROM THE FOUNDATION EXCAVATION PRIOR TO POURING CONCRETE.
- 7. ENGINEER TO BE NOTIFIED IF EXCESSIVE UNDERGROUND WATER IS ENCOUNTERED.
- 8. BACKFILL AND COMPACT FOUNDATION WALLS BELOW GRADE WITH MAXIMUM 2'-0" DIFFERENTIAL IN ELEVATION FROM ONE SIDE TO THE OTHER.

CONCRETE:

1. CONCRETE SHALL CONFORM TO CSA A23.1,2,3 FOR QUALITY CONTROL AND REBAR PLACEMENT.

- 2. REINFORCING STEEL: * CONFORM TO: CAN/CSA-G30.18-M92 CSA-G30.5-M1983 * WELDED WIRE FABRIC SHALL HAVE A MINIMUM YIELD STRENGTH
 - fy = 448 MPa (65,000psi) * REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH
 - fy = 400 MPa (58,000psi)
- 3. ALL REINFORCING BARS SHALL BE SUPPORTED IN THE FORMS AND SPACED WITH STANDARD ACCESSORIES SO THAT THERE IS NO MOVEMENT DURING CONCRETE PLACEMENT.
- 4. REINFORCING IS TO BE GENERALLY PLACED IN ACCORDANCE WITH REINFORCING STEEL INSTITUTE OF CANADA "MANUAL OF STANDARD PRACTICE". ALL SPLICES SHALL BE CLASS "B" IN ACCORDANCE WITH THE FOLLOWING TABLE

CONCRETE	SPLICE	LENGTH: n	nm (")	
REBAR SIZE	25MPa	30MPa	32MPa	35MPa
10M	400	400	400	400
	(16")	(16")	(16")	(16")
15M	600	600	500	500
	(24")	(24")	(20")	(20")
20M	800	700	700	700
	(32")	(28")	(28")	(28")
25M	1200	1100	1100	1100
	(48")	(44")	(44")	(44")

5. THE ABOVE TABLE ALSO APPLIES TO ALL DOWELS UNLESS NOTED OTHERWISE.

6. ALL HORIZONTAL BARS SHALL BE HOOKED 600mm (24") AROUND CORNERS/WALL INTERSECTIONS UNLESS NOTED OTHERWISE. 7. PROVIDE 1-15M PER REINFORCEMENT LAYER OR CONCRETE LAYER AROUND ALL WINDOW, DOOR, MECHANICAL OR SIMILAR OPENINGS

EXTENDING 24" (min.) BEYOND EACH CORNER OF THE OPENING.

8. THE CLEAR DISTANCE BETWEEN REINFORCING STEEL AND SURFACE OF CONCRETE SHALL BE AS FOLLOWS:

	EXPOSURE CLASS				
EXPOSURE CONDITION	N	F-1,F-2,S-1,S-2	C-XL.C-1,C-3,A-1,A-2,A-3		
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH		75mm 3"	75mm 3"		
BEAMS, GIRDERS, COLUMNS, AND PILES	30mm 1-1/4"	40mm 1-1/2"	50mm 2"		
SLABS, WALLS, JOISTS, SHELLS, AND FOLDED PLATES	20mm 3/4"	40mm 1-1/2"	60mm 2-3/8"		
RATIO OF COVER TO NOMINAL BAR DIAMETER	1.0	1.5	2.0		
RATIO OF COVER TO NOMINAL MAXIMUM AGGREGATE SIZE	1.0	1.5	2.0		

9. UNLESS OTHERWISE SPECIFIED BY THE OWNER, REINFORCEMENT SHALL BE PLACED WITHIN THE FOLLOWING TOLERANCES: a) CONCRETE COVER: +/-12mm (1/2") BUT NO LESS THAN 1/3 OF THE SPECIFIED MINIMUM COVER b) WHERE THE DEPTH OF THE FLEXURAL MEMBER, THICKNESS OF A WALL, OR THE SMALLEST DIMENSION OF A COLUMN IS

- i) 200mm (8") OR LESS: +/- 8mm (1/4") ii) LARGER THAN 200mm (8") BUT LESS THAN 600mm (24"): 12mm (1/2")
- iii) 600mm OR LARGER: +/-20mm (3/4") c) LATERAL SPACING OF BARS: +/-30mm (1-1/4")
- a) LONGITUDINAL LOCATION OF BENDS AND ENDS OF BARS: +/-50mm (2")
 e) LONGITUDINAL LOCATION OF BENDS AND ENDS AT DISCONTINUOUS ENDS OF MEMBERS: +/- 20mm (3/4")
- 10. ALL REINFORCING STEEL IN PLACE TO BE MADE AVAILABLE FOR INSPECTION BY ENGINEER BEFORE POURING THE CONCRETE. ENGINEER
- TO BE NOTIFIED WELL IN ADVANCE OF POURING SCHEDULE. ENGINEER TO CARRY OUT INSPECTION AT HIS DISCRETION. 11. ALL GRIT, MUD AND DEBRIS SHALL BE REMOVED FROM THE REINFORCING STEEL PRIOR TO THE PLACEMENT OF CONCRETE.
- 12. PROVIDE PORTLAND CEMENT OF CANADIAN MANUFACTURE CONFORMING WITH CSA/CAN 3-A5, TYPE 10.
- 13. CONTROL JOINTS IN SLABS ON GRADE SHALL BE 1/4 THE THICKNESS OF THE SLAB. SPACING OF THE CONTROL JOINTS IN SLABS ON GRADE
- SHALL NOT EXCEED THE GREATER OF 30 TIMES THE THICKNESS OF THE SLAB OR 15'-0". 14. PROVIDE CLEAN, UNCOATED SAND AND COARSE AGGREGATES FROM APPROVED SOURCES WHICH CONFORM WITH CSA/CAN 3-A2M. REFER
- TO THE CONCRETE SPECIFICATIONS PROVIDED BELOW FOR AGGREGATE SIZE.
- 15. PROVIDE PLASTICIZER OF WATER REDUCING ADD MIXTURE WHERE INCREASED WORKABILITY IS REQUIRED. DO NOT ADD WATER ON-SITE. 16. USE HIGH FREQUENCY VIBRATION TO PLACE ALL CONCRETE.
- 17. ALL CONCRETE SHALL HAVE A 3" SLUMP +/-1".
- 18. TAKE ADEQUATE MEASURES TO PROTECT CONCRETE FROM EXPOSURE TO FREEZING TEMPERATURES AT LEAST SEVEN DAYS AFTER
- CONCRETE PLACEMENT
- 19. ALL CONCRETE SHALL BE TESTED BY A CSA CERTIFIED CONCRETE TESTING LABORATORY.
 - i) CURING TYPE 1 3 DAYS @ 10°C TO ATTAIN 40% OF THE SPECIFIED STRENGTH.
 - ii) CURING TYPE 2 7 DAYS @ 10°C TO ATTAIN 70% OF THE SPECIFIED STRENGTH.
 - iii) CURING TYPE 3 A WET CURING PERIOD OF 7 DAYS.

20. CONCRETE SPECIFICATIONS

DESCRIPTION	CLASS OF CONCRETE	STRENGTH @ 28 DAYS	W/CM RATIO	AIR ENTRAINMENT	CURING TYPE	AGGREGATE
FOOTINGS	Ν	20MPa	N/A	N/A	1	14-20mm
FOUNDATION WALLS	F-2	25MPa	0.55	4-7%	1	14-20mm
INTERIOR / EXTERIOR SLABS	C-2	32MPa	0.45	5-8%	2	14-20mm

NOTES:			NORTH					
 @ AT c/w COMPLETE WITH Ø DIAMETER 	OHD PROJ. P.T.	OJ. PROJECTION		DESIGN	TVG	No.	REVISION DESCRIPTION	MM/DD/YY
FRR FIRE-RESISTANCE RATING	r/w	REINFORCED WITH		DRAWN	TVG	1.	PRELIMINARY - FOR DISCUSSION ONLY	11/22/23
FDN FOUNDATION e/w EACH WAY	SG T&G	SLIDE GATE TOUNGE AND GROOVE	Y	CHECKED	MR	2.	ISSUED FOR MINOR VARIANCE	05/16/24
EX. EXISTING H HIGH/HORIZONTAL	t/o TYP.	TOP OF TYPICAL		APPROVE	D MR			
HR HOUR LLV LONG LEG VERTICAL	u/s V	UNDERSIDE VERTICAL		DATE	MAY 2024			
LVL LAMINATED VANEER LUMB max. MAXIMUM	ER w/ W	WITH WIDE						
min. MINIMUM OBC ONTARIO BUILDING CODE	WWM	WELDED WIRE MESH						
o/c ON CENTER								

LUMBER

- 1. LUMBER SHALL BE No. 2 GRADE SPECIES SPF OR BETTER UNLESS NOTED OTHERWISE.
- 2. ALL POSTS SHALL BE No.1 GRADE SPECIES SPF OR BETTER UNLESS NOTED OTHERWISE.
- 3. ALL WOOD IN CONTACT WITH CONCRETE OR SOIL/EARTH SHALL BE PRESSURE TREATED. 4. ALL PLYWOOD SHALL CONFORM TO CSA STANDARD 0121 OR 0151.
- 5. ALL OSB SHALL CONFORM TO CSA 0325 CONSTRUCTION SHEATHING OR CSA 0437.0 WAFERBOARD WITH
- MINIMUM PANEL MARK FOR 0325 OSB 1R24/2F16. 6. ALL NAILS/FASTENERS USED WITH PRESSURE TREATED LUMBER SHALL BE GALVANIZED OR ACQ QUALIFIED.
- 7. ALL MANUFACTURED CONNECTORS/HARDWARE TO BE GALVANIZED.
- ALL NAILING SHALL BE AS PER NBCC/OBC UNLESS NOTED OTHERWISE. NAILS AND SPIKES SHALL CONFORM TO THE CSA STANDARD B111 "WIRE NAILS, SPIKES AND STAPLES".
- 9. ALL BOLTS USED FOR WOOD CONNECTIONS SHALL BE A307 (min.). WHERE BOLTS ARE EXPOSED TO
- CORROSIVE ENVIRONMENTS, A307 (min.) HOT DIPPED GALVANIZED BOLTS, NUTS, WASHERS SHALL BE USED. 10. STUD WALLS SHALL BE ANCHORED TO THE FOUNDATION WITH 1/2"Ø ANCHOR BOLTS SPACED AS INDICATED

TRUSSES:

ON THE DRAWINGS.

- 1. THE ROOF TRUSSES ARE TO BE DESIGNED FOR THE SPECIFIED LOADS. THE SUPPLIER IS TO PROVIDE ERECTION AND MEMBER FABRICATION DRAWINGS STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. THE DRAWINGS MUST INDICATE DESIGN LOADS, TIMBER SPECIES, GRADES, BRACING AND CONNECTORS. ALL TRUSSES MUST BE ANCHORED WITH APPROPRIATE TIE-DOWN METAL ANCHORS TO RESIST UPLIFT AS CALCULATED AND SHOWN IN THE TRUSS DESIGN CALCULATIONS AND TRUSS DRAWINGS.
- 2. THE BEARING SHOWN ON THE DRAWINGS IS THE MAXIMUM WIDTH TO BE PROVIDED AND THE TRUSS MANUFACTURER MUST DESIGN THE TRUSSES TO SUIT THE BEARING WIDTH.
- 3. ALL PERMANENT BRACING FOR TRUSSES SHALL BE SECURELY ANCHORED BY BACK BRACING DIAGONALLY OR ATTACHING TO ENDWALLS ACCORDING TO GUIDELINES PUBLISHED BY THE CANADIAN WOOD TRUSS ASSOCIATION.
- 4. UNBALANCED LOADING CONDITIONS SHALL BE INCLUDED IN THE TRUSS DESIGN. 5. TRUSS DESIGNER TO ACCOUNT FOR INCREASE SNOW LOADS DUE TO ROOF VALLEYS AND SNOW
- SHADOWS/DRIFTING.
- LOADS AND ASSUMPTIONS SHALL BE NOTED ON THE SHOP DRAWINGS. STRUCTURAL STEEL:
- 1. STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF CSA SPECIFICATIONS S16.1 (LATEST EDITION) AND CSA SPECIFICATIONS G40.20 AND G40.21, TYPE 350W (LATEST EDITION) FOR BEAMS AND 350W FOR HSS (CLASS C).
- 2. BOLTED CONNECTIONS SHALL BE MADE USING FRICTION TYPE CONNECTION AND HIGH TENSILE STRENGTH BOLTS (A325 min.). WHERE BOLTS ARE EXPOSED TO CORROSIVE ENVIRONMENTS, A325 (min.) HOT DIPPED GALVANIZED BOLTS, NUTS, WASHERS SHALL BE USED.
- 3. STRUCTURAL STEEL FABRICATOR TO BE CERTIFIED BY THE CANADIAN WELDING BUREAU.
- 4. THE WELDING SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD W59 (LATEST EDITION) AND THE WELDING COMPANY AND WELDERS SHALL BE QUALIFIED UNDER THE REQUIREMENTS OF CSA STANDARD W57 (LATEST EDITION) FOR THE APPROPRIATE POSITION.
- 5. STEEL SHALL BE THOROUGHLY CLEANED AND HOT DIPPED GALVANIZED. AREAS AFFECTED BY WEATHERING, DAMAGE DUE TO HANDLING ETC., SHALL HAVE THE RUST REMOVED AND BE "TOUCHED UP" IN THE FIELD. 6. NO SPLICES IN BEAMS AND COLUMNS WILL BE ALLOWED WITHOUT WRITTEN APPROVAL OF THE ENGINEER. BUTT
- WELDS IN SPLICES AND MOMENT CONNECTIONS MUST BE ULTRASONICALLY TESTED OR X-RAYED AND PASSED BY AN INDEPENDENT TESTING COMPANY.
- 7. DO NOT CUT OPENINGS IN STRUCTURAL STEEL MEMBERS WITHOUT THE ENGINEER'S APPROVAL. 8. THE CONTRACTOR/STEEL SUPPLIER SHALL PROVIDE AND REMOVE AFTERWARDS ALL TEMPORARY BRACING
- NECESSARY TO KEEP THE STRUCTURE TRUE AND PLUMB. ALL TEMPORARY BRACING SHALL ACCOUNT FOR CONSTRUCTION AND ENVIRONMENTAL LOADS DURING CONSTRUCTION.

6. TRUSS SHOP DRAWINGS SHALL BE PROVIDED TO THE ENGINEER TO REVIEW PRIOR TO FABRICATION. DESIGN

9. CONTRACTOR TO VERIFY SITE MEASUREMENTS PRIOR TO FABRICATION OF STEEL.

DESIGN INFORMATION:

SB-1 LOCATION (PARKHILL, ON - 15 km+/- FROM BUILDING LOCATION) BUILDING LOCATION (10098 KLONDYKE ROAD, GRAND BEND, ON) <u>ROOF</u> LIVE LOAD

> SNOW (Ss) RAIN (Sr)

SPECIFIED BALANCED SNOW LOAD (S) = [Ss x Cb] + Sr TRUSS DESIGN DEAD LOADS TOP CHORD DEAD LOAD BOTTOM CHORD DEAD LOAD

Cb

<u>WIND PRESSURE</u> q 1/50 YEAR

q 1/10 YEAR WIND DESIGN CATEGORY TERRAIN NET WIND UPLIFT

NET UNBALANCED UPLIFT SITE CLASSIFICATION (ASSUMED) NOTE: ALL DESIGN LOADS ARE UNFACTORED

0.4 kPa (8.4 psf) 0.55 1.72 kPa (35.9 psf) 0.15 kPa (3.1 psf) <u>0.35 kPa (7.3 psf)</u> 0.50 kPa (10.4 psf 0.49 kPa (10.2 psf) 0.38 kPa (7.9 psf) CATEGORY 2

2.4 kPa (50.1 psf)

OPEN 0.96 kPa (20.0 psf) 0.46 kPa (9.6 psf)

MUNICIPALITY OF LAMBTON SHORES ZONING BY-LAW 1-2003 RESIDENTIAL - 6 (R6) ZONE (SCHEDULE 'A-1') DETAIL LOT AREA PROPERTY FRONTAGE INTERIOR SIDE YARD REAR YARD LOT COVERAGE BUILDING AREA BUILDING HEIGHT





UNDARY

KLONDYKE ROAD

